STORMWATER REPORT

Juniper Avenue – Taxlot 81021CB01500 Preliminary Subdivision Plan Warrenton, OR 97146

Prepared November 23, 2020 Revised October 27, 2021 Revised December 7, 2021

Prepared By:



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Nearest Floodplain FIRMette

HydraFlow Express output

HydroCAD output

Firwood Design Group, Inc.

I. OBJECTIVE

The objective is to address stormwater system capacity for the Juniper Avenue Subdivision, as well as addressing how fill will impact the existing storm sewer outfall and stormwater storage in the existing wetlands.

II. STORMWATER STORAGE IN EXISTING WETLANDS & FLOODPLAIN

The proposed fill slope will not encroach on the existing wetlands and so will not impact stormwater storage within the wetlands. There are no mapped FEMA floodplains in the project vicinity (FIRMette included in the appendix for reference). The entire project site is mapped as Zone X, area of minimal flood hazard. The mapped 100-year floodplain nearest to the site is roughly 1/3 of a mile to the northeast, mapped at an elevation of 8 feet (FIRMette included in the appendix for reference). Fill is planned to be placed down to an elevation of approximately 17 feet, well above the nearest 100-year floodplain elevation. Therefore, stormwater storage within wetland and floodplain limits will not be negatively impacted.

III. EXISTING STORMWATER OUTFALL & CONNECTION

The existing storm sewer outfall line is a 12" corrugated plastic pipe, assumed to be ADS N-12, constructed at a slope of approximately 4.6% with a hydraulic capacity of 8.9 cfs will be abandoned. A new 15" PVC pipe will be re-routed through our site to avoid encumbering the site with a 20' utility easement that would be required by the City of Warrenton. To demonstrate that the 15" PVC storm line is capable of conveying runoff from all existing, proposed, and possible future tributary impervious areas, a maximum tributary impervious area was calculated using HydroCAD. A pipe capacity analysis for a 15" pipe with a 1.9% slope shows a maximum capacity of 8.9 cfs and matches the existing 12" corrugated plastic pipe. As shown in the HydroCAD model, it takes approximately 6.3 acres of impervious surface (with a minimum time of concentration) to generate a peak runoff of 8.9 cfs in a 100-year storm event. The 100-year storm event of 6.1" over 24 hours was modeled using the Santa Barbara Urban Hydrograph (SBUH) with a Type 1A rainfall distribution. HydraFlow Express and HydroCAD outputs are included in the appendix for reference. The existing outfall will be protected until the new storm pipe and swale is installed.

The high point of Juniper Avenue is approximately 750' upstream of the existing Juniper Avenue catch basins; assuming a paved width of 60' at full build-out, there could be up to 1.0 acres of tributary impervious roadway. Coupled with a proposed project site of 1.1 acres (much of which is not impervious), approximately 2/3 of total existing outfall pipe capacity is still available. Therefore, the existing outfall has sufficient capacity.

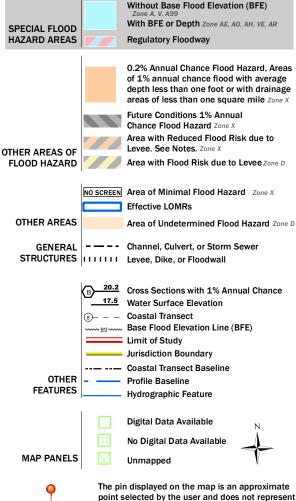
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National Flood Hazard Layer FIRMette



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT



This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

an authoritative property location.

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 6/9/2020 at 11:31:22 AM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

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National Flood Hazard Layer FIRMette

250

500

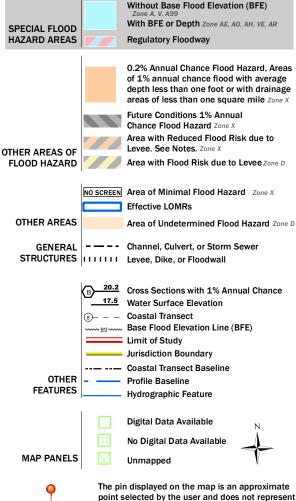
1,000

1,500



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

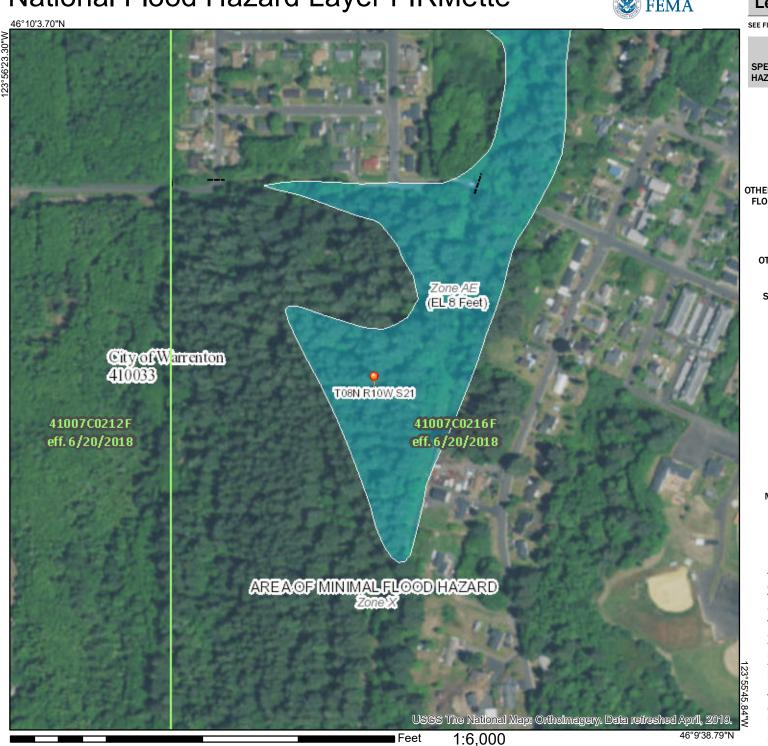


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2,000

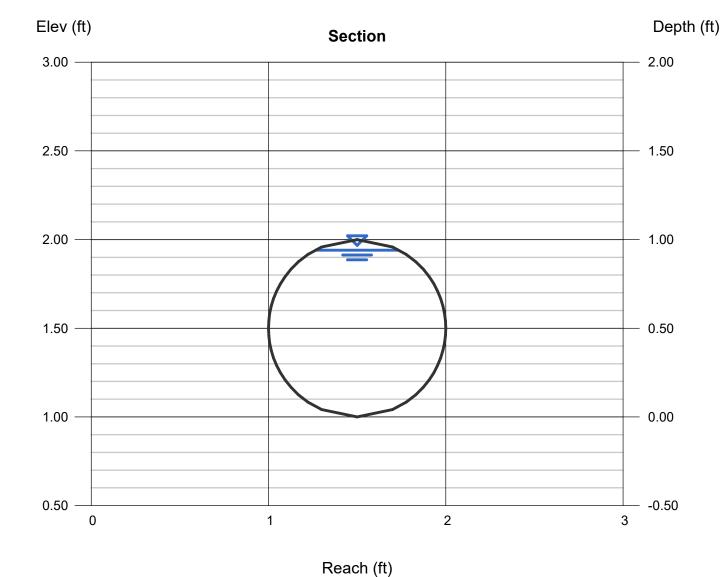
Channel Report

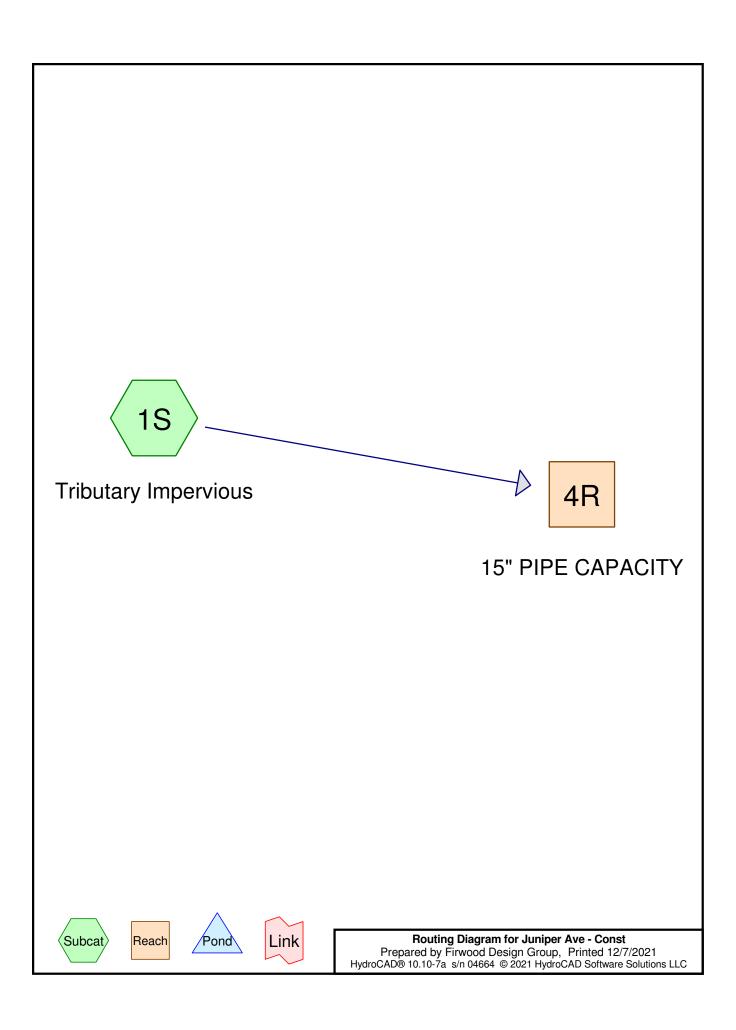
Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Tuesday, Jul 28 2020

12in Outfall Capacity

Circular		Highlighted	
Diameter (ft)	= 1.00	Depth (ft)	= 0.94
. ,		Q (cfs)	= 8.901
		Area (sqft)	= 0.77
Invert Elev (ft)	= 1.00	Velocity (ft/s)	= 11.61
Slope (%)	= 4.60	Wetted Perim (ft)	= 2.65
N-Value	= 0.012	Crit Depth, Yc (ft)	= 0.99
		Top Width (ft)	= 0.47
Calculations		EGL (ft)	= 3.04
Compute by:	Known Depth	` ,	
Known Depth (ft)	= 0.94		
. , ,			





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Rainfall Events Listing (selected events)

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	Warrenton 100-Yr	Type IA 24-hr		Default	24.00	1	6.10	2

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Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
6.200	98	Paved parking, HSG B (1S)
6.200	98	TOTAL AREA

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Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
6.200	HSG B	1S
0.000	HSG C	
0.000	HSG D	
0.000	Other	
6.200		TOTAL AREA

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Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
 (acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
0.000	6.200	0.000	0.000	0.000		Paved parking	1S
0.000	6.200	0.000	0.000	0.000	6.200	TOTAL AREA	

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Pipe Listing (all nodes)

Line#	Node	In-Invert	Out-Invert	Length Slope		n	Width	Diam/Height	Inside-Fill
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)
1	4R	100.00	98.10	100.0	0.0190	0.013	0.0	15.0	0.0

Juniper Ave - Const

Type IA 24-hr Warrenton 100-Yr Rainfall=6.10"

Prepared by Firwood Design Group
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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SBUH method, Split Pervious/Imperv.
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Tributary Impervious Runoff Area=6.200 ac 100.00% Impervious Runoff Depth>5.85" Tc=6.0 min CN=0/98 Runoff=8.88 cfs 3.023 af

Reach 4R: 15" PIPE CAPACITYAvg. Flow Depth=1.02' Max Vel=8.27 fps Inflow=8.88 cfs 3.023 af 15.0" Round Pipe n=0.013 L=100.0' S=0.0190 '/' Capacity=8.90 cfs Outflow=8.88 cfs 3.022 af

Total Runoff Area = 6.200 ac Runoff Volume = 3.023 af Average Runoff Depth = 5.85" 0.00% Pervious = 0.000 ac 100.00% Impervious = 6.200 ac

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Summary for Subcatchment 1S: Tributary Impervious

Runoff 8.88 cfs @ 7.91 hrs, Volume= 3.023 af, Depth> 5.85"

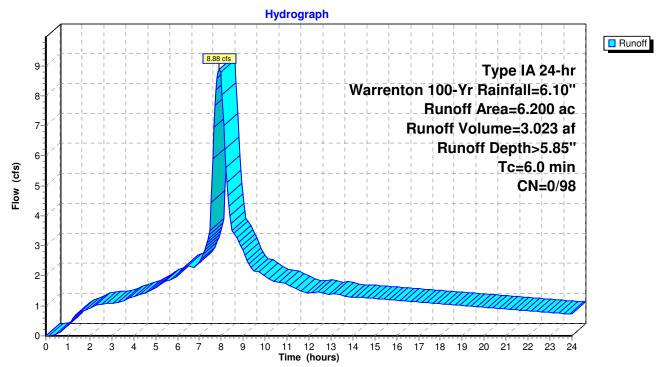
Routed to Reach 4R: 15" PIPE CAPACITY

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type IA 24-hr Warrenton 100-Yr Rainfall=6.10"

_	Area	(ac)	CN	Desc	cription		
	6.	200	98	Pave	ed parking,	HSG B	
	6.	200	98	100.	00% Imper	vious Area	
	Tc (min)	Lengt (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	5.0	,	•	` '	,	, ,	Direct Entry, Standard Min Tc
	5.0		0 T	otal, l	ncreased to	o minimum	Tc = 6.0 min

Total, Increased to minimum Tc = 6.0 min

Subcatchment 1S: Tributary Impervious



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Summary for Reach 4R: 15" PIPE CAPACITY

[52] Hint: Inlet/Outlet conditions not evaluated

Inflow Area = 6.200 ac,100.00% Impervious, Inflow Depth > 5.85" for Warrenton 100-Yr event

Inflow = 8.88 cfs @ 7.91 hrs, Volume= 3.023 af

Outflow = 8.88 cfs @ 7.92 hrs, Volume= 3.022 af, Atten= 0%, Lag= 0.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Max. Velocity= 8.27 fps, Min. Travel Time= 0.2 min Avg. Velocity = 5.19 fps, Avg. Travel Time= 0.3 min

Peak Storage= 107 cf @ 7.92 hrs

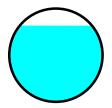
Average Depth at Peak Storage= 1.02', Surface Width= 0.97' Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 8.90 cfs

15.0" Round Pipe

n = 0.013

Length= 100.0' Slope= 0.0190 '/'

Inlet Invert= 100.00', Outlet Invert= 98.10'



Reach 4R: 15" PIPE CAPACITY

